

at a distance of  $0.416 x_u$  (which has been rounded off to  $0.42 x_u$  in the code) from the extreme compression fibre; and the total force of compression is  $0.36 f_{ck} b x_u$ . The lever arm, that is, the distance between the centroid of compressive force and centroid of tensile force is equal to  $(d - 0.416 x_u)$ . Hence the upper limit for the moment of resistance of a singly reinforced rectangular section is given by the following equation:

$$M_{u,lim} = 0.36 f_{ck} b x_{u,max} \times (d - 0.416 x_{u,max})$$

Substituting for  $x_{u,max}$  from Table B and transposing  $f_{ck} b d^2$ , we get the values of the limiting moment of resistance factors for singly reinforced rectangular beams and slabs. These values are given in Table C. The tensile reinforcement percentage,  $p_{t,lim}$  corresponding to the limiting moment of resistance is obtained by equating the forces of tension and compression.

$$\frac{p_{t,lim} b d (0.87 f_y)}{100} = 0.36 f_{ck} b x_{u,max}$$

Substituting for  $x_{u,max}$  from Table B, we get the values of  $p_{t,lim} f_y / f_{ck}$  as given in Table C.

TABLE C LIMITING MOMENT OF RESISTANCE AND REINFORCEMENT INDEX FOR SINGLY REINFORCED RECTANGULAR SECTIONS (Clause 2.3)

| $f_y$ , N/mm <sup>2</sup>        | 250   | 415   | 500   |
|----------------------------------|-------|-------|-------|
| $\frac{M_{u,lim}}{f_{ck} b d^2}$ | 0.149 | 0.138 | 0.133 |
| $\frac{p_{t,lim} f_y}{f_{ck}}$   | 21.97 | 19.82 | 18.87 |

The values of the limiting moment of resistance factor  $M_{u,lim}/bd^2$  for different grades of concrete and steel are given in Table D. The corresponding percentages of reinforcements are given in Table E. These are the maximum permissible percentages for singly reinforced sections.

TABLE D LIMITING MOMENT OF RESISTANCE FACTOR  $M_{u,lim}/bd^2$ , N/mm<sup>2</sup> FOR SINGLY REINFORCED RECTANGULAR SECTIONS (Clause 2.3)

| $f_{ck}$ , N/mm <sup>2</sup> | $f_y$ , N/mm <sup>2</sup> |      |      |
|------------------------------|---------------------------|------|------|
|                              | 250                       | 415  | 500  |
| 15                           | 2.24                      | 2.07 | 2.00 |
| 20                           | 2.98                      | 2.76 | 2.66 |
| 25                           | 3.73                      | 3.45 | 3.33 |
| 30                           | 4.47                      | 4.14 | 3.99 |

TABLE E MAXIMUM PERCENTAGE OF TENSILE REINFORCEMENT  $p_{t,lim}$  FOR SINGLY REINFORCED RECTANGULAR SECTIONS

(Clause 2.3)

| $f_{ck}$ , N/mm <sup>2</sup> | $f_y$ , N/mm <sup>2</sup> |      |      |
|------------------------------|---------------------------|------|------|
|                              | 250                       | 415  | 500  |
| 15                           | 1.32                      | 0.72 | 0.57 |
| 20                           | 1.76                      | 0.96 | 0.76 |
| 25                           | 2.20                      | 1.19 | 0.94 |
| 30                           | 2.64                      | 1.43 | 1.13 |

### 2.3.1 Under-Reinforced Sections

Under-reinforced section means a singly reinforced section with reinforcement percentage not exceeding the appropriate value given in Table E. For such sections, the depth of neutral axis  $x_u$  will be smaller than  $x_{u,max}$ . The strain in steel at the limit state of collapse will, therefore, be more than  $\frac{0.87 f_y}{E_s} + 0.002$  and, the design stress in steel will be  $0.87 f_y$ . The depth of neutral axis is obtained by equating the forces of tension and compression.

$$\frac{p_t b d}{100} (0.87 f_y) = 0.36 f_{ck} b x_u$$

$$\frac{x_u}{d} = \left( \frac{p_t}{100} \right) \frac{0.87 f_y}{0.36 f_{ck}}$$

The moment of resistance of the section is equal to the product of the tensile force and the lever arm.

$$M_u = \frac{p_t b d}{100} (0.87 f_y) (d - 0.416 x_u) = 0.87 f_y \left( \frac{p_t}{100} \right) \left( 1 - 0.416 \frac{x_u}{d} \right) b d^2$$

Substituting for  $\frac{x_u}{d}$  we get

$$M_u = 0.87 f_y \left( \frac{p_t}{100} \right) \times \left[ 1 - 1.005 \frac{f_y}{f_{ck}} \left( \frac{p_t}{100} \right) \right] b d^2$$

2.3.1.1 Charts 1 to 18 have been prepared by assigning different values to  $M_u/b$  and plotting  $d$  versus  $p_t$ . The moment values in the charts are in units of kN.m per metre width. Charts are given for three grades of steel and two grades of concrete, namely M 15 and M 20, which are most commonly used for flexural members. Tables 1 to 4 cover a wider range, that is, five values of  $f_y$  and four grades of concrete up to M 30. In these tables, the values of percentage of reinforcement  $p_t$  have been tabulated against  $M_u/bd^2$ .

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**U. H. Varyani**



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