

at a distance of $0.416 x_u$ (which has been rounded off to $0.42 x_u$ in the code) from the extreme compression fibre; and the total force of compression is $0.36 f_{ck} b x_u$. The lever arm, that is, the distance between the centroid of compressive force and centroid of tensile force is equal to $(d - 0.416 x_u)$. Hence the upper limit for the moment of resistance of a singly reinforced rectangular section is given by the following equation:

$$M_{u,lim} = 0.36 f_{ck} b x_{u,max} \times (d - 0.416 x_{u,max})$$

Substituting for $x_{u,max}$ from Table B and transposing $f_{ck} b d^2$, we get the values of the limiting moment of resistance factors for singly reinforced rectangular beams and slabs. These values are given in Table C. The tensile reinforcement percentage, $p_{t,lim}$ corresponding to the limiting moment of resistance is obtained by equating the forces of tension and compression.

$$\frac{p_{t,lim} b d (0.87 f_y)}{100} = 0.36 f_{ck} b x_{u,max}$$

Substituting for $x_{u,max}$ from Table B, we get the values of $p_{t,lim} f_y / f_{ck}$ as given in Table C.

TABLE C LIMITING MOMENT OF RESISTANCE AND REINFORCEMENT INDEX FOR SINGLY REINFORCED RECTANGULAR SECTIONS (Clause 2.3)

f_y , N/mm ²	250	415	500
$\frac{M_{u,lim}}{f_{ck} b d^2}$	0.149	0.138	0.133
$\frac{p_{t,lim} f_y}{f_{ck}}$	21.97	19.82	18.87

The values of the limiting moment of resistance factor $M_{u,lim}/bd^2$ for different grades of concrete and steel are given in Table D. The corresponding percentages of reinforcements are given in Table E. These are the maximum permissible percentages for singly reinforced sections.

TABLE D LIMITING MOMENT OF RESISTANCE FACTOR $M_{u,lim}/bd^2$, N/mm² FOR SINGLY REINFORCED RECTANGULAR SECTIONS (Clause 2.3)

f_{ck} , N/mm ²	f_y , N/mm ²		
	250	415	500
15	2.24	2.07	2.00
20	2.98	2.76	2.66
25	3.73	3.45	3.33
30	4.47	4.14	3.99

TABLE E MAXIMUM PERCENTAGE OF TENSILE REINFORCEMENT $p_{t,lim}$ FOR SINGLY REINFORCED RECTANGULAR SECTIONS

(Clause 2.3)

f_{ck} , N/mm ²	f_y , N/mm ²		
	250	415	500
15	1.32	0.72	0.57
20	1.76	0.96	0.76
25	2.20	1.19	0.94
30	2.64	1.43	1.13

2.3.1 Under-Reinforced Sections

Under-reinforced section means a singly reinforced section with reinforcement percentage not exceeding the appropriate value given in Table E. For such sections, the depth of neutral axis x_u will be smaller than $x_{u,max}$. The strain in steel at the limit state of collapse will, therefore, be more than $\frac{0.87 f_y}{E_s} + 0.002$ and, the design stress in steel will be $0.87 f_y$. The depth of neutral axis is obtained by equating the forces of tension and compression.

$$\frac{p_t b d}{100} (0.87 f_y) = 0.36 f_{ck} b x_u$$

$$\frac{x_u}{d} = \left(\frac{p_t}{100} \right) \frac{0.87 f_y}{0.36 f_{ck}}$$

The moment of resistance of the section is equal to the product of the tensile force and the lever arm.

$$M_u = \frac{p_t b d}{100} (0.87 f_y) (d - 0.416 x_u) = 0.87 f_y \left(\frac{p_t}{100} \right) \left(1 - 0.416 \frac{x_u}{d} \right) b d^2$$

Substituting for $\frac{x_u}{d}$ we get

$$M_u = 0.87 f_y \left(\frac{p_t}{100} \right) \times \left[1 - 1.005 \frac{f_y}{f_{ck}} \left(\frac{p_t}{100} \right) \right] b d^2$$

2.3.1.1 Charts 1 to 18 have been prepared by assigning different values to M_u/b and plotting d versus p_t . The moment values in the charts are in units of kN.m per metre width. Charts are given for three grades of steel and two grades of concrete, namely M 15 and M 20, which are most commonly used for flexural members. Tables 1 to 4 cover a wider range, that is, five values of f_y and four grades of concrete up to M 30. In these tables, the values of percentage of reinforcement p_t have been tabulated against M_u/bd^2 .

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Table of Contents Sp16 Code Civil Engineering

1. Understanding the eBook Sp16 Code Civil Engineering
 - The Rise of Digital Reading Sp16 Code Civil Engineering
 - Advantages of eBooks Over Traditional Books
2. Identifying Sp16 Code Civil Engineering
 - Exploring Different Genres
 - Considering Fiction vs. Non-Fiction
 - Determining Your Reading Goals
3. Choosing the Right eBook Platform
 - Popular eBook Platforms
 - Features to Look for in an Sp16 Code Civil Engineering
 - User-Friendly Interface
4. Exploring eBook Recommendations from Sp16 Code Civil Engineering
 - Personalized Recommendations
 - Sp16 Code Civil Engineering User Reviews and Ratings

- Sp16 Code Civil Engineering and Bestseller Lists
- 5. Accessing Sp16 Code Civil Engineering Free and Paid eBooks
 - Sp16 Code Civil Engineering Public Domain eBooks
 - Sp16 Code Civil Engineering eBook Subscription Services
 - Sp16 Code Civil Engineering Budget-Friendly Options
- 6. Navigating Sp16 Code Civil Engineering eBook Formats
 - ePub, PDF, MOBI, and More
 - Sp16 Code Civil Engineering Compatibility with Devices
 - Sp16 Code Civil Engineering Enhanced eBook Features
- 7. Enhancing Your Reading Experience
 - Adjustable Fonts and Text Sizes of Sp16 Code Civil Engineering
 - Highlighting and Note-Taking Sp16 Code Civil Engineering
 - Interactive Elements Sp16 Code Civil Engineering
- 8. Staying Engaged with Sp16 Code Civil Engineering
 - Joining Online Reading Communities
 - Participating in Virtual Book Clubs
 - Following Authors and Publishers Sp16 Code Civil Engineering
- 9. Balancing eBooks and Physical Books Sp16 Code Civil Engineering
 - Benefits of a Digital Library
 - Creating a Diverse Reading Collection Sp16 Code Civil Engineering
- 10. Overcoming Reading Challenges
 - Dealing with Digital Eye Strain
 - Minimizing Distractions
 - Managing Screen Time
- 11. Cultivating a Reading Routine Sp16 Code Civil Engineering
 - Setting Reading Goals Sp16 Code Civil Engineering
 - Carving Out Dedicated Reading Time
- 12. Sourcing Reliable Information of Sp16 Code Civil Engineering
 - Fact-Checking eBook Content of Sp16 Code Civil Engineering
 - Distinguishing Credible Sources

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 - Utilizing eBooks for Skill Development
 - Exploring Educational eBooks
14. Embracing eBook Trends
 - Integration of Multimedia Elements
 - Interactive and Gamified eBooks

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